



TRANSVERSE VIBRATION OF A DAMAGED CIRCULAR ANNULAR PLATE WITH A FREE EDGE

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1. INTRODUCTION

The present study deals with the determination of the fundamental frequency of transverse vibration of a circular, annular plate simply supported or clamped at the outer edge and free at the inner edge (see Figure 1). It is assumed that the structural element, originally isotropic, experiences a damaging process in such a manner that the subdomain defined by $b \leq r \leq c$ acquires polarly anisotropic characteristics while the remaining portion conserves its original, virgin constitutive properties.

The fundamental eigenvalue is determined using the strain energy damage detection method as developed by Cornwell *et al.* [1] and following the modification proposed in reference [2]. It is important to point out that the fundamentals of the methodology were originally proposed by Stubbs *et al.* [3].

2. ANALYTICAL TREATMENT OF THE PROBLEM

Referring to the system shown in Figure 1 its transverse normal modes are governed by the well-known functional

$$\begin{aligned}
 J(W) = D \int_b^c \left[\left(W'' + \frac{W'}{\bar{r}} \right)^2 - 2(1 - \nu) \frac{W'W''}{\bar{r}} \right] \bar{r} \, d\bar{r} \\
 + \int_c^a \left[D_r W''^2 + D_\theta \left(\frac{W'}{\bar{r}} \right)^2 + 2D_r \nu_\theta \frac{W'W''}{\bar{r}} \right] \bar{r} \, d\bar{r} - \rho h \omega^2 \int_b^a W^2 \bar{r} \, d\bar{r} \quad (1)
 \end{aligned}$$

subject to the boundary conditions

$$W''(b) + \frac{\nu}{b} W'(b) = 0, \quad W'''(b) + \frac{W''(b)}{b} - \frac{W'(b)}{b^2} = 0, \quad (2a, b)$$

$$W(a) = 0, \quad W''(a) + \frac{\nu_\theta}{a} W'(a) = 0 \quad \text{or} \quad W'(a) = 0. \quad (3a-c)$$

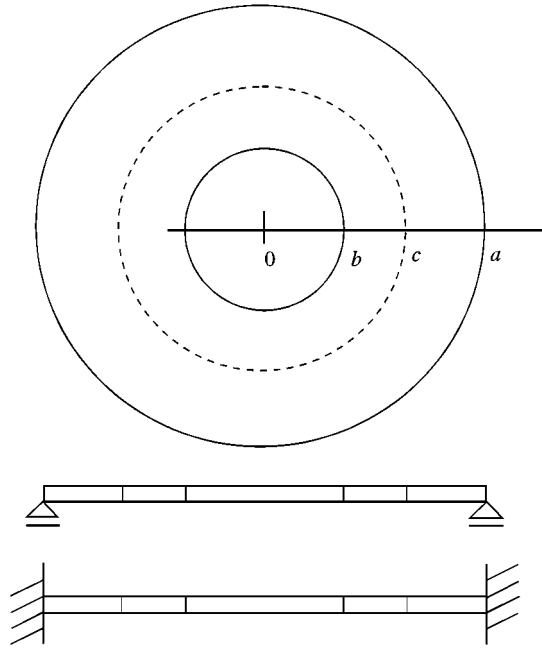


Figure 1. Vibrating structural system under study.

Introducing the dimensionless variable $r = \bar{r}/a$ and defining

$$r_b = b/a, \quad r_c = c/a \quad (4a, b)$$

one obtains, substituting in equations (1)–(3),

$$\begin{aligned} \frac{a^2}{D} J(W) = & \int_{r_b}^{r_c} \left[\left(W'' + \frac{W'}{r} \right)^2 + 2(1 - \nu) \frac{W'W''}{r} \right] r \, dr \\ & + \int_{r_c}^1 \left[\frac{D_r}{D} W''^2 + \frac{D_\theta}{D} \frac{W'^2}{r^2} + 2 \frac{D_r}{D} \nu_\theta \frac{W'W''}{r} \right] r \, dr - \Omega^2 \int_{r_b}^1 W^2 r \, dr, \end{aligned} \quad (5)$$

$$W''(r_b) + \frac{\nu}{r_b} W'(r_b) = 0, \quad W'''(r_b) + \frac{W''(r_b)}{r_b} - \frac{W'(r_b)}{r_b^2} = 0, \quad (6a, b)$$

$$W(1) = 0, \quad W''(1) + \nu_\theta W'(1) = 0 \quad \text{or} \quad W'(1) = 0 \quad (7a, b, c)$$

where

$$\Omega^2 = \frac{\rho h a^4}{D} \omega^2.$$

Substituting the approximating function

$$W \cong W_a = \sum_{j=1}^N C_j \varphi_j(r)$$

in the dimensionless functional (5) and applying Ritz' minimization conditions one obtains the following homogeneous linear system of equations:

$$\begin{aligned} \frac{a^2}{2D} \frac{\partial J}{\partial C_i} = \sum_{j=1}^N \left\{ \int_{r_b}^{r_c} \left[\left(\varphi_j'' + \frac{\varphi_j'}{r} \right) \left(\varphi_i'' + \frac{\varphi_i'}{r} \right) - \frac{1-\nu}{r} (\varphi_j' \varphi_i' + \varphi_j' \varphi_i'') \right] r \, dr \right. \\ + \int_{r_c}^1 \left[\frac{D_r}{D} \varphi_j'' \varphi_i'' + \frac{D_\theta}{D} \frac{\varphi_j' \varphi_i'}{r^2} + \frac{D_r}{D} \nu_0 \frac{\varphi_j'' \varphi_i' + \varphi_j' \varphi_i''}{r} \right] r \, dr \\ \left. - \Omega^2 \int_{r_b}^1 \varphi_j \varphi_i r \, dr \right\} C_j = 0, \quad i = 1, \dots, N. \end{aligned} \tag{9}$$

The following co-ordinate functions are employed:

$$\varphi_j(r) = a_j r^{p+j-1} + b_j r^{j+2} + c_j r^{j+1} + 1, \tag{10}$$

where the coefficients are evaluated by substituting each polynomial in the governing boundary conditions with the exception of equation (2b) which was conveniently disregarded since the Rayleigh-Ritz method does not require satisfaction of the natural boundary conditions. The exponential parameter "p" constitutes Rayleigh's optimization variable.

3. NUMERICAL RESULTS

The numerical determinations were performed making $\nu = \nu_0 = 0.3$, $D_r/D = 1$ and $D_\theta/D = 0.50, 0.75, 1, 1.25$ and 1.50 and taking $N = 7$.

In order to verify the accuracy of the approach it was first applied to the fully isotropic plate where very accurate results have recently been obtained following the exact method [4]. The agreement is remarkably good (see Table 1).

Table 2 depicts values of Ω_1 in the case of a damaged annular plate when the outer boundary is simply supported. The fundamental eigenvalues are obtained for $r_b = 0.2, 0.4$ and 0.6 and $r_c = 0.2, 0.4, \dots, 1$.

Table 3 deals with situation where the outer boundary is clamped.

The algorithmic procedure did yield an excellent rate of convergence in all the cases considered.

TABLE 1
Fundamental frequency coefficients of isotropic, annular plates

Outer BC		r_b		
		0.2	0.4	0.6
Simply supported	(1)	4.718	4.764	5.711
	(2)	4.7177	4.7640	5.7107
Clamped	(1)	10.409	13.603	25.674
	(2)	10.4080	13.6027	25.6742

Note: (1) Present approximate approach; (2) Exact solution [4].

TABLE 2

Fundamental frequency coefficients of vibrating isotropic-orthotropic plates (outer simply supported boundary)

D_0/D	r_b	r_c				
		0.2	0.4	0.6	0.8	1
0.50	0.2	3.370	3.974	4.278	4.525	4.718
	0.4	—	3.244	4.029	4.462	4.764
	0.6	—	—	3.844	5.028	5.711
0.75	0.2	4.135	4.363	4.504	4.623	4.718
	0.4	—	4.085	4.412	4.616	4.764
	0.6	—	—	4.870	5.380	5.711
1	0.2	4.718	4.718	4.718	4.718	4.718
	0.4	—	4.764	4.764	4.764	4.764
	0.6	—	—	5.711	5.711	5.711
1.25	0.2	5.197	5.047	4.923	4.811	4.718
	0.4	—	5.343	5.092	4.907	4.764
	0.6	—	—	6.439	6.023	5.711
1.50	0.2	5.608	5.353	5.119	4.902	4.718
	0.4	—	5.854	5.399	5.046	4.764
	0.6	—	—	7.089	6.319	5.711

TABLE 3

Fundamental frequency coefficients of vibrating isotropic-orthotropic plates (outer clamped boundary)

D_0/D	r_b	r_c				
		$r_c = 0.2$	0.4	0.6	0.8	1
0.50	0.2	9.132	9.188	10.224	10.383	10.409
	0.4	—	12.696	13.295	13.557	13.603
	0.6	—	—	25.052	25.558	25.674
0.75	0.2	9.827	10.150	10.317	10.396	10.409
	0.4	—	13.163	13.450	13.580	13.603
	0.6	—	—	25.366	25.616	25.674
1	0.2	10.409	10.409	10.409	10.409	10.409
	0.4	—	13.603	13.603	13.603	13.603
	0.6	—	—	25.674	25.674	25.674
1.25	0.2	10.911	10.660	10.500	10.422	10.409
	0.4	—	14.018	13.754	13.626	13.603
	0.6	—	—	25.977	25.732	25.674
1.50	0.2	11.352	10.904	10.590	10.435	10.409
	0.4	—	14.411	13.903	13.648	13.603
	0.6	—	—	26.275	25.790	25.674

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